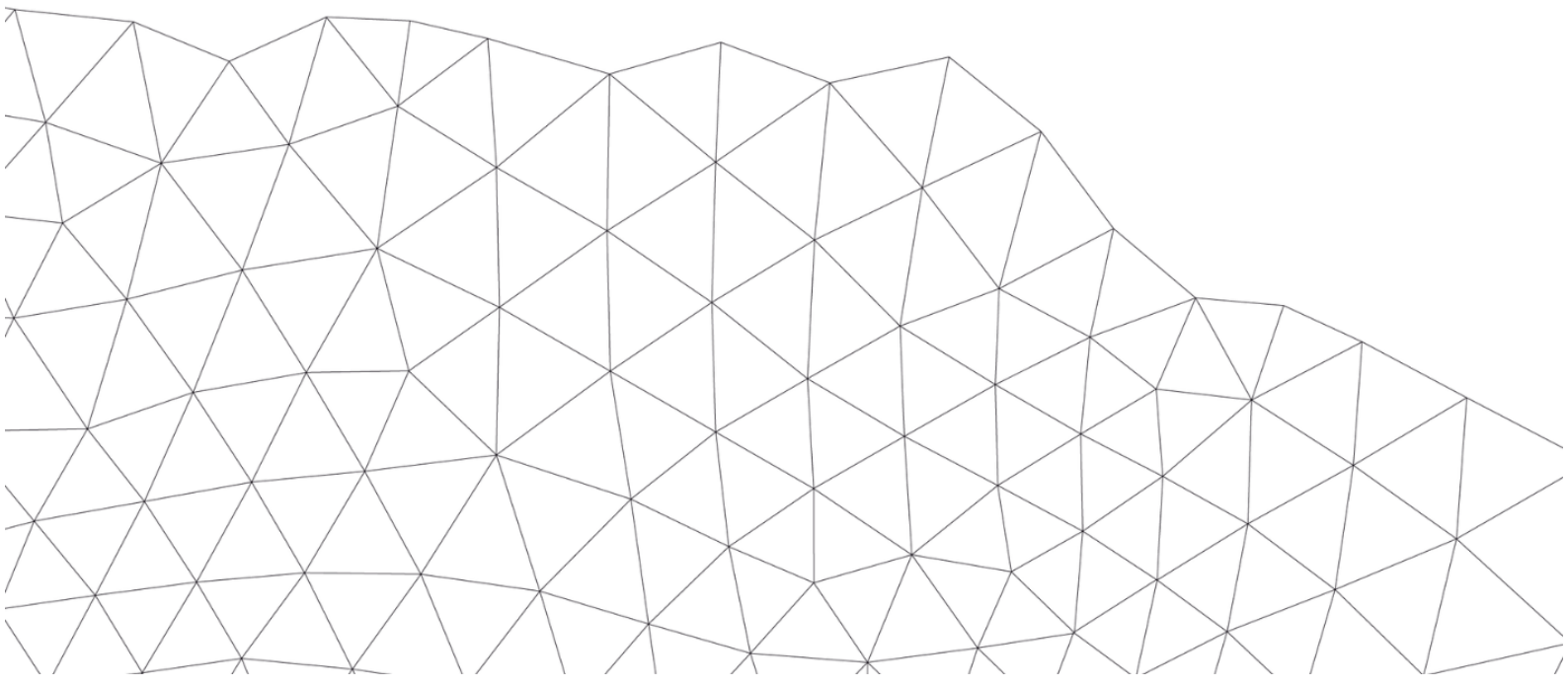


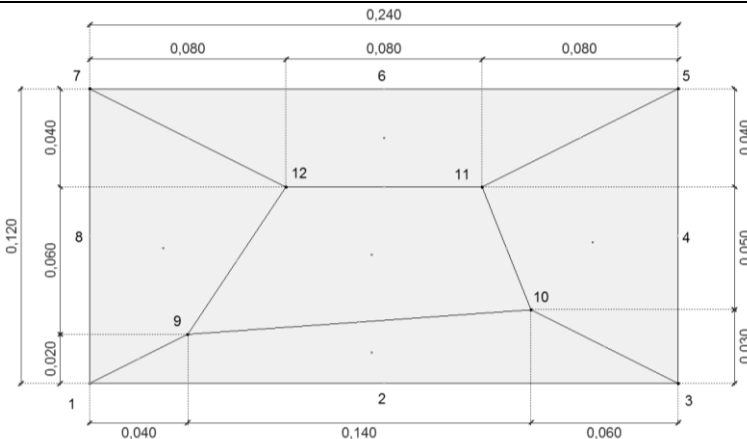


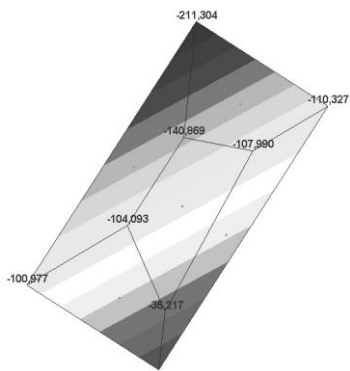
AXISVM

PATCH TESTS

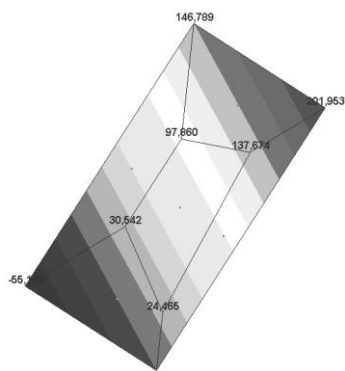
2023



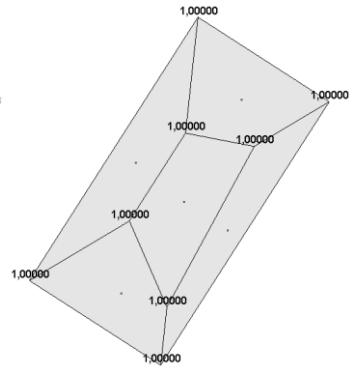
| Topic | Rigid body motion | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|-------------------------|---|-------------------------|-----------|------------|-----------|------------|----------|----------|-----|------------|---------|---------|------|-------------------|---------|---------|-----|---|------|---|------|---|---|------|------|---|------|------|----|------|------|---|------|------|----|------|------|---|------|------|----|------|------|
| Analysis Type | Non-linear static | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Geometry |  <p> $t = 1 \text{ mm}$ $a = 240 \text{ mm}$ $b = 120 \text{ mm}$ </p> <table border="1" data-bbox="710 929 1284 1265"> <thead> <tr> <th>Node</th> <th>x [m]</th> <th>y [m]</th> <th>Node</th> <th>x [m]</th> <th>y [m]</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0</td> <td>0</td> <td>7</td> <td>0</td> <td>0,12</td> </tr> <tr> <td>2</td> <td>0,12</td> <td>0</td> <td>8</td> <td>0</td> <td>0,06</td> </tr> <tr> <td>3</td> <td>0,24</td> <td>0</td> <td>9</td> <td>0,04</td> <td>0,02</td> </tr> <tr> <td>4</td> <td>0,24</td> <td>0,06</td> <td>10</td> <td>0,18</td> <td>0,03</td> </tr> <tr> <td>5</td> <td>0,24</td> <td>0,12</td> <td>11</td> <td>0,16</td> <td>0,08</td> </tr> <tr> <td>6</td> <td>0,12</td> <td>0,12</td> <td>12</td> <td>0,08</td> <td>0,08</td> </tr> </tbody> </table> | Node | x [m] | y [m] | Node | x [m] | y [m] | 1 | 0 | 0 | 7 | 0 | 0,12 | 2 | 0,12 | 0 | 8 | 0 | 0,06 | 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 |
| Node | x [m] | y [m] | Node | x [m] | y [m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 0 | 0 | 7 | 0 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 0,12 | 0 | 8 | 0 | 0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loads | Prescribed displacement: $\varphi_z = 1,0$ radian at node 1 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Boundary Conditions | $e_x = e_y = e_z = \varphi_x = \varphi_y = 0$ at node 1 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Material Properties | $E = 100 \text{ kN / cm}^2$ $\rho = 1000 \text{ kg / m}^3$ $\nu = 0,25$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Element types | Shell elements | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Target | Check displacements of node 3 and prove that all stresses are zero. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Results | <p>Reference: Richard H. MacNeal and Robert L. Harder, "A Proposed Standard Set of Problems to Test Finite Element Accuracy", Finite Elements in Analysis and Design 1, pp. 3-20, 1985.</p> <table border="1" data-bbox="383 1769 1396 1971"> <thead> <tr> <th>Displacements of node 3</th> <th>AxisVM</th> <th>Analytical</th> <th>error [%]</th> </tr> </thead> <tbody> <tr> <td>e_x [mm]</td> <td>-110,327</td> <td>-110,327</td> <td>0,0</td> </tr> <tr> <td>e_y [mm]</td> <td>201,953</td> <td>201,953</td> <td>0,0</td> </tr> <tr> <td>φ_z [rad]</td> <td>1,00000</td> <td>1,00000</td> <td>0,0</td> </tr> </tbody> </table> | Displacements of node 3 | AxisVM | Analytical | error [%] | e_x [mm] | -110,327 | -110,327 | 0,0 | e_y [mm] | 201,953 | 201,953 | 0,0 | φ_z [rad] | 1,00000 | 1,00000 | 0,0 | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Displacements of node 3 | AxisVM | Analytical | error [%] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| e_x [mm] | -110,327 | -110,327 | 0,0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| e_y [mm] | 201,953 | 201,953 | 0,0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| φ_z [rad] | 1,00000 | 1,00000 | 0,0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



e_x [mm]



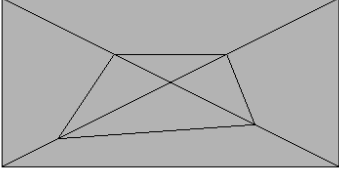
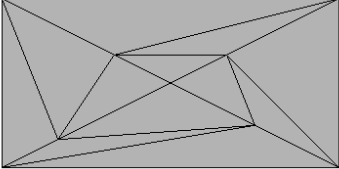
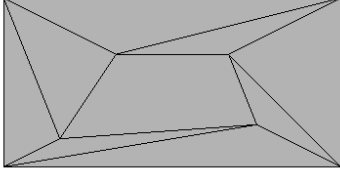
e_y [mm]



ϕ_z [rad]

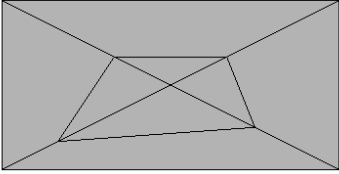
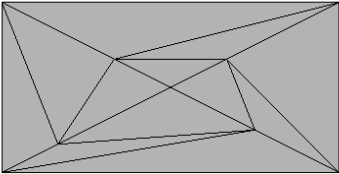
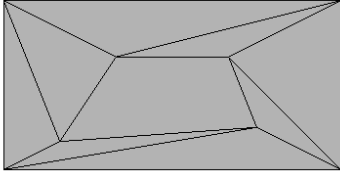
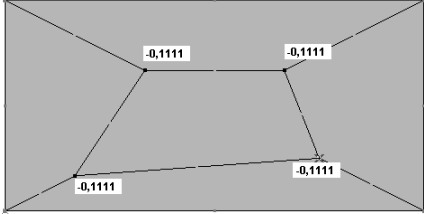
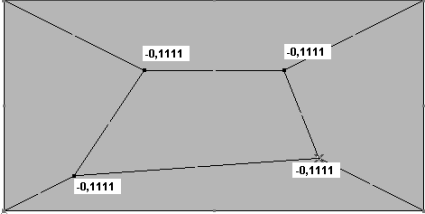
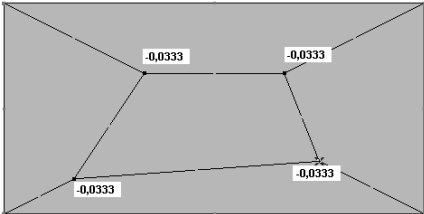
All the stresses are zero up to eleven digits.

| Topic | Patch test – membrane plate | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---------------------|---|-----------|-----------|-----------|-------|-------|-------|---|------|------|---|------|------|---|------|------|---|------|------|---|------|------|---|------|------|---|------|------|----|------|------|---|------|------|----|------|------|---|------|------|----|------|------|
| Analysis Type | Linear static | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Geometry | <div style="text-align: center;"> </div> <p style="margin-left: 40px;"> $t = 1 \text{ mm}$ $a = 240 \text{ mm}$ $b = 120 \text{ mm}$ </p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>Node</th> <th>x [m]</th> <th>y [m]</th> <th>Node</th> <th>x [m]</th> <th>y [m]</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0</td> <td>0</td> <td>7</td> <td>0</td> <td>0,12</td> </tr> <tr> <td>2</td> <td>0,12</td> <td>0</td> <td>8</td> <td>0</td> <td>0,06</td> </tr> <tr> <td>3</td> <td>0,24</td> <td>0</td> <td>9</td> <td>0,04</td> <td>0,02</td> </tr> <tr> <td>4</td> <td>0,24</td> <td>0,06</td> <td>10</td> <td>0,18</td> <td>0,03</td> </tr> <tr> <td>5</td> <td>0,24</td> <td>0,12</td> <td>11</td> <td>0,16</td> <td>0,08</td> </tr> <tr> <td>6</td> <td>0,12</td> <td>0,12</td> <td>12</td> <td>0,08</td> <td>0,08</td> </tr> </tbody> </table> | Node | x [m] | y [m] | Node | x [m] | y [m] | 1 | 0 | 0 | 7 | 0 | 0,12 | 2 | 0,12 | 0 | 8 | 0 | 0,06 | 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 |
| Node | x [m] | y [m] | Node | x [m] | y [m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 0 | 0 | 7 | 0 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 0,12 | 0 | 8 | 0 | 0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loads | <p>Prescribed displacements: $e_x = x + y/2$ $e_y = y + x/2$</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>Node</th> <th>e_x [m]</th> <th>e_y [m]</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0</td> <td>0</td> </tr> <tr> <td>2</td> <td>0,12</td> <td>0,06</td> </tr> <tr> <td>3</td> <td>0,24</td> <td>0,12</td> </tr> <tr> <td>4</td> <td>0,27</td> <td>0,18</td> </tr> <tr> <td>5</td> <td>0,30</td> <td>0,24</td> </tr> <tr> <td>6</td> <td>0,18</td> <td>0,18</td> </tr> <tr> <td>7</td> <td>0,06</td> <td>0,12</td> </tr> <tr> <td>8</td> <td>0,03</td> <td>0,06</td> </tr> </tbody> </table> | Node | e_x [m] | e_y [m] | 1 | 0 | 0 | 2 | 0,12 | 0,06 | 3 | 0,24 | 0,12 | 4 | 0,27 | 0,18 | 5 | 0,30 | 0,24 | 6 | 0,18 | 0,18 | 7 | 0,06 | 0,12 | 8 | 0,03 | 0,06 | | | | | | | | | | | | | | | |
| Node | e_x [m] | e_y [m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 0 | 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 0,12 | 0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 0,24 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 0,27 | 0,18 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 0,30 | 0,24 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 0,18 | 0,18 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 7 | 0,06 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 8 | 0,03 | 0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Boundary Conditions | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Material Properties | $E = 100 \text{ kN / cm}^2$ $\rho = 1000 \text{ kg / m}^3$ $\nu = 0,25$ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---------------------|--|--------------|-----------------------------------|---------------------|--------------|---------------------|--|--|--------------|--------------|-----------------|--------------|--------------|-----------------|---|---------|---------|--------|---------|---------|--------|----|---------|---------|--------|---------|---------|--------|----|---------|---------|--------|---------|---------|--------|----|---------|---------|--------|---------|---------|--------|------|-----------------------------------|--|----------------------|--|-----------|-----------|-----------|-----------|---|------|------|------|------|----|-------|------|-------|------|----|------|------|------|------|----|------|------|------|------|
| Element types | <p>Shell elements – 4 mesh cases</p> <div style="display: flex; justify-content: space-around; align-items: center;"> <div style="text-align: center;">  <p>rectangular and triangular elements mixed</p> </div> <div style="text-align: center;">  <p>triangular elements only</p> </div> <div style="text-align: center;">  <p>rectangular elements only</p> </div> </div> | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Target | Determine forces and displacements of inner nodes. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Results | <p>Reference:</p> <p>Richard H. MacNeal and Robert L. Harder, "A Proposed Standard Set of Problems to Test Finite Element Accuracy", Finite Elements in Analysis and Design 1, pp. 3-20, 1985.</p> <p><u>Forces</u></p> <table border="1" data-bbox="438 1093 1347 1462"> <thead> <tr> <th rowspan="2">Node</th> <th colspan="3">AxisVM results in four mesh cases</th> <th colspan="3">Analytical solution</th> </tr> <tr> <th>n_x [kN/m]</th> <th>n_y [kN/m]</th> <th>n_{xy} [kN/m]</th> <th>n_x [kN/m]</th> <th>n_y [kN/m]</th> <th>n_{xy} [kN/m]</th> </tr> </thead> <tbody> <tr> <td>9</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> </tr> <tr> <td>10</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> </tr> <tr> <td>11</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> </tr> <tr> <td>12</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> <td>1333,33</td> <td>1333,33</td> <td>400,00</td> </tr> </tbody> </table> <p><u>Displacements</u></p> <table border="1" data-bbox="588 1559 1193 1888"> <thead> <tr> <th rowspan="2">Node</th> <th colspan="2">AxisVM results in four mesh cases</th> <th colspan="2">Analytical solutions</th> </tr> <tr> <th>e_x [m]</th> <th>e_y [m]</th> <th>e_x [m]</th> <th>e_y [m]</th> </tr> </thead> <tbody> <tr> <td>9</td> <td>0,05</td> <td>0,04</td> <td>0,05</td> <td>0,04</td> </tr> <tr> <td>10</td> <td>0,195</td> <td>0,12</td> <td>0,195</td> <td>0,12</td> </tr> <tr> <td>11</td> <td>0,20</td> <td>0,16</td> <td>0,20</td> <td>0,16</td> </tr> <tr> <td>12</td> <td>0,12</td> <td>0,12</td> <td>0,12</td> <td>0,12</td> </tr> </tbody> </table> <p>Calculated displacements and forces are the same in each mesh case as the results in AxisVM.</p> | Node | AxisVM results in four mesh cases | | | Analytical solution | | | n_x [kN/m] | n_y [kN/m] | n_{xy} [kN/m] | n_x [kN/m] | n_y [kN/m] | n_{xy} [kN/m] | 9 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | 10 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | 11 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | 12 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | Node | AxisVM results in four mesh cases | | Analytical solutions | | e_x [m] | e_y [m] | e_x [m] | e_y [m] | 9 | 0,05 | 0,04 | 0,05 | 0,04 | 10 | 0,195 | 0,12 | 0,195 | 0,12 | 11 | 0,20 | 0,16 | 0,20 | 0,16 | 12 | 0,12 | 0,12 | 0,12 | 0,12 |
| Node | AxisVM results in four mesh cases | | | Analytical solution | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | n_x [kN/m] | n_y [kN/m] | n_{xy} [kN/m] | n_x [kN/m] | n_y [kN/m] | n_{xy} [kN/m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 9 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 10 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 11 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 12 | 1333,33 | 1333,33 | 400,00 | 1333,33 | 1333,33 | 400,00 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Node | AxisVM results in four mesh cases | | Analytical solutions | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | e_x [m] | e_y [m] | e_x [m] | e_y [m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 9 | 0,05 | 0,04 | 0,05 | 0,04 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 10 | 0,195 | 0,12 | 0,195 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 11 | 0,20 | 0,16 | 0,20 | 0,16 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 12 | 0,12 | 0,12 | 0,12 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Software Release Number: X7r1c
 Date: 19. 04. 2023.
 Tested by: InterCAD
 Page number:
 File name: PatchMacNeel_Bending plate.axs

| Topic | Constant curvature patch test – bending plate | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---------------------|--|-------------------|-------------------|-------------------|-------------------|-------|-------|---|---|---|--------|------|-------|---|--------|------|-------|---|--------|------|-------|---|--------|------|-------|---|--------|------|-------|------|--------|------|-------|------|--------|------|-------|---|------|------|----|------|------|
| Analysis Type | Linear static | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Geometry | <p> $t = 1 \text{ mm}$ $a = 240 \text{ mm}$ $b = 120 \text{ mm}$ </p> <table border="1"> <thead> <tr> <th>Node</th> <th>x [m]</th> <th>y [m]</th> <th>Node</th> <th>x [m]</th> <th>y [m]</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0</td> <td>0</td> <td>7</td> <td>0</td> <td>0,12</td> </tr> <tr> <td>2</td> <td>0,12</td> <td>0</td> <td>8</td> <td>0</td> <td>0,06</td> </tr> <tr> <td>3</td> <td>0,24</td> <td>0</td> <td>9</td> <td>0,04</td> <td>0,02</td> </tr> <tr> <td>4</td> <td>0,24</td> <td>0,06</td> <td>10</td> <td>0,18</td> <td>0,03</td> </tr> <tr> <td>5</td> <td>0,24</td> <td>0,12</td> <td>11</td> <td>0,16</td> <td>0,08</td> </tr> <tr> <td>6</td> <td>0,12</td> <td>0,12</td> <td>12</td> <td>0,08</td> <td>0,08</td> </tr> </tbody> </table> | Node | x [m] | y [m] | Node | x [m] | y [m] | 1 | 0 | 0 | 7 | 0 | 0,12 | 2 | 0,12 | 0 | 8 | 0 | 0,06 | 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 |
| Node | x [m] | y [m] | Node | x [m] | y [m] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 0 | 0 | 7 | 0 | 0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 0,12 | 0 | 8 | 0 | 0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 0,24 | 0 | 9 | 0,04 | 0,02 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 0,24 | 0,06 | 10 | 0,18 | 0,03 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 0,24 | 0,12 | 11 | 0,16 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 0,12 | 0,12 | 12 | 0,08 | 0,08 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Loads | - | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Boundary Conditions | Prescribed displacements: <table border="1"> <thead> <tr> <th>Node</th> <th>e_z [m]</th> <th>φ_x [rad]</th> <th>φ_y [rad]</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>2</td> <td>0,0072</td> <td>0,06</td> <td>-0,12</td> </tr> <tr> <td>3</td> <td>0,0288</td> <td>0,12</td> <td>-0,24</td> </tr> <tr> <td>4</td> <td>0,0378</td> <td>0,18</td> <td>-0,27</td> </tr> <tr> <td>5</td> <td>0,0504</td> <td>0,24</td> <td>-0,30</td> </tr> <tr> <td>6</td> <td>0,0216</td> <td>0,18</td> <td>-0,18</td> </tr> <tr> <td>7</td> <td>0,0072</td> <td>0,12</td> <td>-0,06</td> </tr> <tr> <td>8</td> <td>0,0018</td> <td>0,06</td> <td>-0,03</td> </tr> </tbody> </table> | Node | e_z [m] | φ_x [rad] | φ_y [rad] | 1 | 0 | 0 | 0 | 2 | 0,0072 | 0,06 | -0,12 | 3 | 0,0288 | 0,12 | -0,24 | 4 | 0,0378 | 0,18 | -0,27 | 5 | 0,0504 | 0,24 | -0,30 | 6 | 0,0216 | 0,18 | -0,18 | 7 | 0,0072 | 0,12 | -0,06 | 8 | 0,0018 | 0,06 | -0,03 | | | | | | |
| Node | e_z [m] | φ_x [rad] | φ_y [rad] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 0 | 0 | 0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 0,0072 | 0,06 | -0,12 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 0,0288 | 0,12 | -0,24 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 0,0378 | 0,18 | -0,27 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 0,0504 | 0,24 | -0,30 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 0,0216 | 0,18 | -0,18 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 7 | 0,0072 | 0,12 | -0,06 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 8 | 0,0018 | 0,06 | -0,03 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

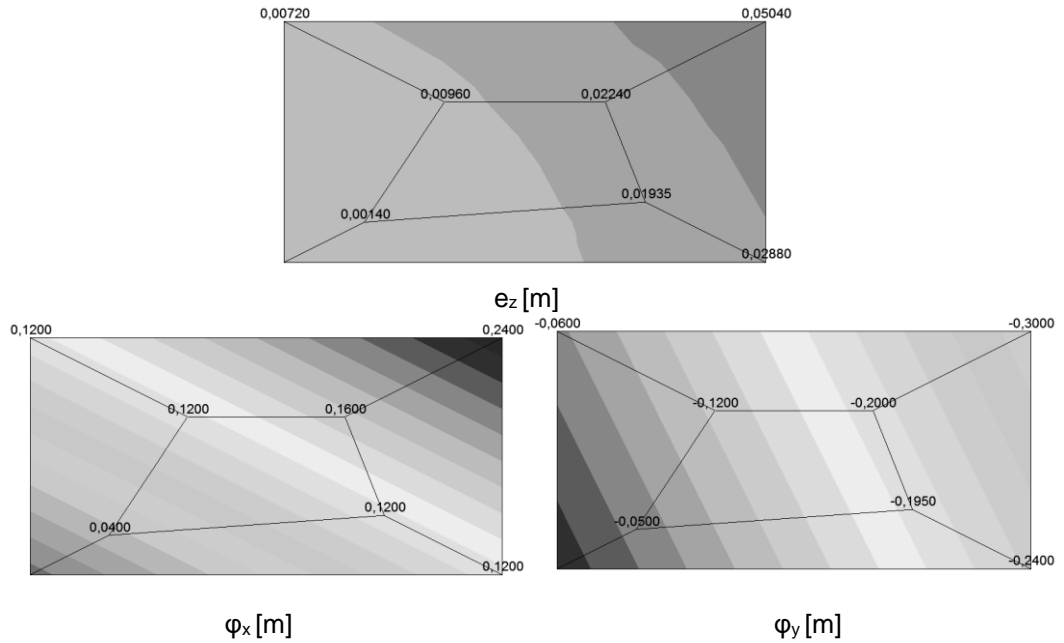
| | |
|---------------------|---|
| Material Properties | $E = 100 \text{ kN / cm}^2$ $\rho = 1000 \text{ kg / m}^3$ $\nu = 0,25$ |
| Element types | <p>Shell elements – 4 mesh cases:</p> <div style="display: flex; justify-content: space-around; align-items: center;"> <div style="text-align: center;">  <p>rectangular and triangular elements mixed</p> </div> <div style="text-align: center;">  <p>triangular elements only</p> </div> <div style="text-align: center;">  <p>rectangular elements only</p> </div> </div> |
| Target | Determine moments and displacements of inner nodes. |
| Results | <p>Reference:</p> <p>Richard H. MacNeal and Robert L. Harder, "A Proposed Standard Set of Problems to Test Finite Element Accuracy", Finite Elements in Analysis and Design 1, pp. 3-20, 1985.</p> <p><u>Moments</u></p> <p>Analytical solution at each inner node: $m_x = m_y = -0,1111 \text{ kNmm/m}$ $m_{xy} = -0,0333 \text{ kNmm/m}$</p> <p>Results in AxisVM:</p> <div style="display: flex; justify-content: space-around; align-items: center;"> <div style="text-align: center;">  <p>$m_x \text{ [kNmm/m]}$</p> </div> <div style="text-align: center;">  <p>$m_y \text{ [kNmm/m]}$</p> </div> </div> <div style="text-align: center; margin-top: 20px;">  <p>$m_{xy} \text{ [kNmm/m]}$</p> </div> |

Displacements

Analytical solution:

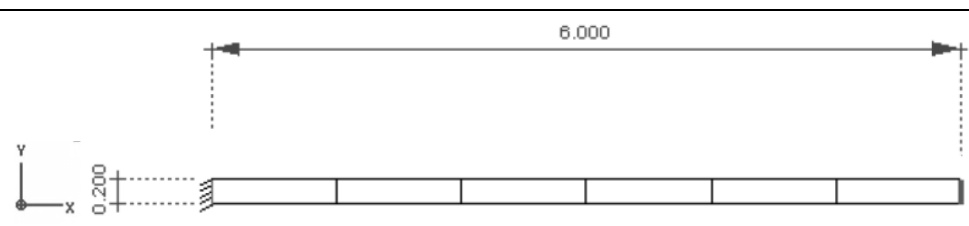
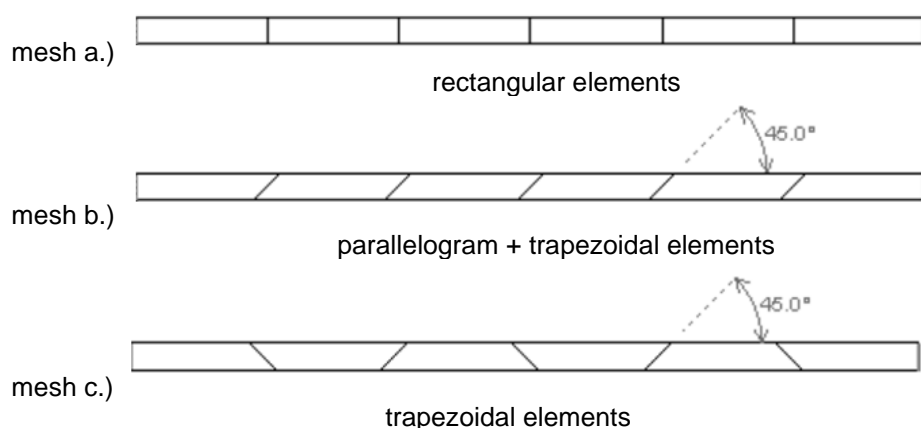
| Node | x [m] | y [m] | e_z [m] | φ_x [rad] | φ_y [rad] |
|------|-------|-------|-----------|-------------------|-------------------|
| 9 | 0,04 | 0,02 | 0,00140 | 0,0400 | 0,0500 |
| 10 | 0,18 | 0,03 | 0,01935 | 0,1200 | 0,1950 |
| 11 | 0,16 | 0,08 | 0,02240 | 0,1600 | 0,2000 |
| 12 | 0,08 | 0,08 | 0,00960 | 0,1200 | 0,1200 |

Results in AxisVM:



Calculated displacements and moments are the same in each mesh case as the results in AxisVM.

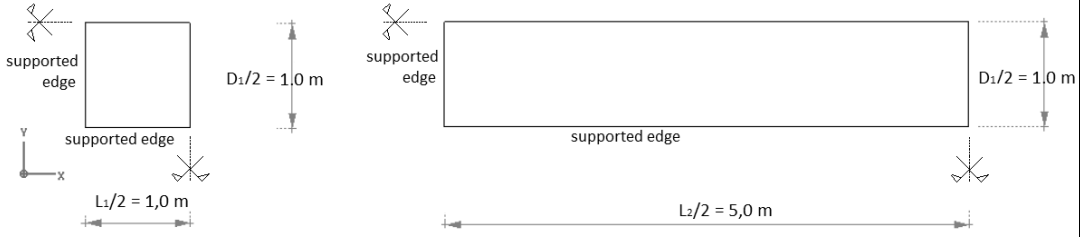
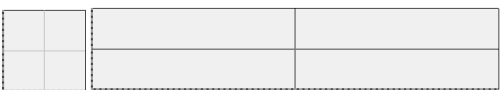
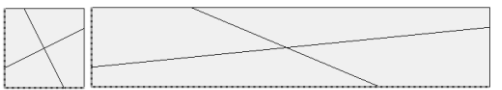
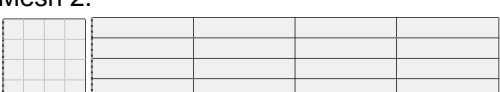

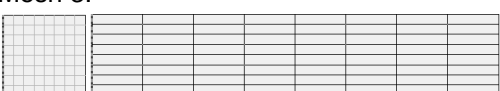
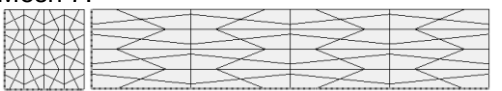
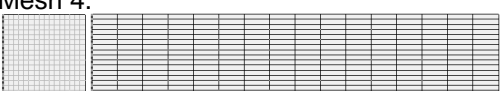
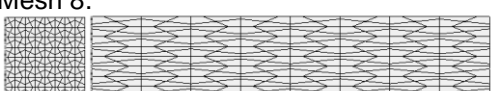
Software Release Number: X7r1c
 Date: 19. 04. 2023.
 Tested by: InterCAD
 Page number:
 File name: Cantilever.axs

| | |
|---------------------|---|
| Topic | In-plane and out-of-plane shear and bending patch test of shell element |
| Analysis Type | Linear static |
| Geometry |  <p style="text-align: center;">length: $h = 6,0$ m width: $w = 0,20$ m depth: $d = 0,10$ m</p> |
| Loads | Unit forces on the free end of the beam, each as a different load case: A unit force in y direction, distributed on the edge: 5 kN/m A unit force in z direction, distributed on the edge: 5 kN/m A unit moment about axis x |
| Boundary Conditions | Left edge is clamped: $e_x = e_y = e_z = \phi_x = \phi_y = \phi_z = 0$ |
| Material Properties | $E = 1000$ kN / cm ² $\nu = 0,30$ |
| Element types | Shell elements – 3 mesh cases:  |
| Target | Determine the displacements of the free end of the beam. |
| Results | <p>Analytical solution:</p> <p>Reference: Richard H. MacNeal and Robert L. Harder, "A Proposed Standard Set of Problems to Test Finite Element Accuracy", Finite Elements in Analysis and Design 1, pp. 3-20, 1985.</p> <p style="padding-left: 40px;">$e_y = 0,1081$ m $e_z = 0,4321$ m $\phi_x = 0,03411$ rad *</p> |

| | Mesh case | AxisVM results | Analytical solution | e [%] |
|----------------------|-----------|----------------|---------------------|--------|
| e_y [mm] | a | 108,087 | 108,1 | -0,01 |
| | b | 108,015 | | -0,08 |
| | c | 105,716 | | -2,21 |
| e_z [mm] | a | 428,189 | 432,1 | -0,91 |
| | b | 428,743 | | -0,78 |
| | c | 427,531 | | -1,06 |
| φ_x [rad] | a | 0,03012 | 0,03411* | -11,7 |
| | b | 0,03006 | | -11,87 |
| | c | 0,03011 | | -11,73 |

* In our opinion, the φ_x rotation result for torsion is the following:

$$\varphi_x = \int \frac{M_x}{GI_x} dx = 0,03411 [\text{rad}]$$

| | |
|---------------------|---|
| Topic | Shell element test |
| Analysis Type | Linear static |
| Geometry | <p>The analyzed rectangular plates are of 2*2 and 2*10 meters. Only one quarter of the plates are modeled:</p>  <p style="text-align: center;">length: $L_1 = 2,0$ m, $L_2 = 10,0$ m width: $D_1 = D_2 = 2,0$ m depth: $t_1 = t_2 = 0,01$ m</p> |
| Loads | <p>Case 1: Distributed load on the whole plate: $0,1$ kN/m² Case 2: Point load in z direction in the center of the plate: $0,4$ kN ($0,1$ kN on the modeled quarter plate)</p> |
| Boundary Conditions | <p>At the left and bottom edge of the modeled plate: Case 1: clamped edge: $e_x = e_y = e_z = \varphi_x = \varphi_y = \varphi_z = 0$ Case 2: simple support: $e_x = e_y = e_z = \varphi_z = 0$ In each case - because only one quarter of the original plate is modeled – symmetry conditions are applied to the symmetry lines.</p> |
| Material Properties | <p>$E = 1747,2$ kN / cm² $\nu = 0,30$</p> |
| Element types | <p>Shell elements: arrangement of orthogonal and distorted elements, with different mesh density:</p> <div style="display: flex; flex-wrap: wrap;"> <div style="width: 50%;"> <p>Mesh 1.</p>  </div> <div style="width: 50%;"> <p>Mesh 5.</p>  </div> <div style="width: 50%;"> <p>Mesh 2.</p>  </div> <div style="width: 50%;"> <p>Mesh 6.</p>  </div> <div style="width: 50%;"> <p>Mesh 3.</p>  </div> <div style="width: 50%;"> <p>Mesh 7.</p>  </div> <div style="width: 50%;"> <p>Mesh 4.</p>  </div> <div style="width: 50%;"> <p>Mesh 8.</p>  </div> </div> |

| Target | Determine the deflection in z direction in the center of the plate. | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|-------------------|---|--------------------------------|--------------------------------|-------------------|-------|--|------------------|--|-------------------|--|-------|-------|-------|-------|-----------------|------|------|------|------|----------------|------|-------|-------|-------|-----------------|--|--------------------------------|--|--|--|--|--|------------------|--|-------------------|--|--|--|-------|-------|-------|-------|------------------|--|------|------|------|------|------------|---|------|------|------|------|---|------|------|------|------|---|------|------|------|------|---|------|------|------|------|-------------------|--|------|------|------|------|----------------|--|--------------------------------|--|--|--|--|--|------------------|--|-------------------|--|--|--|-------|-------|-------|-------|------------------|--|------|-------|-------|-------|------------|---|------|-------|-------|-------|---|------|-------|-------|-------|---|------|-------|-------|-------|---|------|-------|-------|-------|-------------------|--|------|----|------|------|
| Results | <p>Reference:</p> <p>Richard H. MacNeal and Robert L. Harder, "A Proposed Standard Set of Problems to Test Finite Element Accuracy", Finite Elements in Analysis and Design 1, pp. 3-20, 1985.</p> <table border="1"> <thead> <tr> <th rowspan="3">Analytic results</th> <th colspan="4">e_z deflection [mm]</th> </tr> <tr> <th colspan="2">Distributed load</th> <th colspan="2">Concentrated load</th> </tr> <tr> <th>b/a=1</th> <th>b/a=5</th> <th>b/a=1</th> <th>b/a=5</th> </tr> </thead> <tbody> <tr> <td>Clamped support</td> <td>1,26</td> <td>2,56</td> <td>5,60</td> <td>7,23</td> </tr> <tr> <td>Simple support</td> <td>4,06</td> <td>12,97</td> <td>11,60</td> <td>16,96</td> </tr> </tbody> </table> <p>Results in AxisVM</p> <table border="1"> <thead> <tr> <th colspan="2">Clamped support</th> <th colspan="4">e_z deflection [mm]</th> </tr> <tr> <th colspan="2"></th> <th colspan="2">Distributed load</th> <th colspan="2">Concentrated load</th> </tr> <tr> <th colspan="2"></th> <th>b/a=1</th> <th>b/a=5</th> <th>b/a=1</th> <th>b/a=5</th> </tr> </thead> <tbody> <tr> <td colspan="2">Analytic results</td> <td>1,26</td> <td>2,56</td> <td>5,60</td> <td>7,23</td> </tr> <tr> <td rowspan="4">Mesh cases</td> <td>1</td> <td>1,24</td> <td>2,62</td> <td>5,40</td> <td>6,31</td> </tr> <tr> <td>2</td> <td>1,26</td> <td>2,61</td> <td>5,57</td> <td>7,07</td> </tr> <tr> <td>3</td> <td>1,27</td> <td>2,60</td> <td>5,61</td> <td>7,10</td> </tr> <tr> <td>4</td> <td>1,27</td> <td>2,61</td> <td>5,62</td> <td>7,24</td> </tr> <tr> <td colspan="2">Error of last row</td> <td>0,8%</td> <td>1,9%</td> <td>0,4%</td> <td>0,1%</td> </tr> </tbody> </table> <table border="1"> <thead> <tr> <th colspan="2">Simple support</th> <th colspan="4">e_z deflection [mm]</th> </tr> <tr> <th colspan="2"></th> <th colspan="2">Distributed load</th> <th colspan="2">Concentrated load</th> </tr> <tr> <th colspan="2"></th> <th>b/a=1</th> <th>b/a=5</th> <th>b/a=1</th> <th>b/a=5</th> </tr> </thead> <tbody> <tr> <td colspan="2">Analytic results</td> <td>4,06</td> <td>12,97</td> <td>11,60</td> <td>16,96</td> </tr> <tr> <td rowspan="4">Mesh cases</td> <td>1</td> <td>4,17</td> <td>12,79</td> <td>11,70</td> <td>16,78</td> </tr> <tr> <td>2</td> <td>4,09</td> <td>12,30</td> <td>11,64</td> <td>16,33</td> </tr> <tr> <td>3</td> <td>4,08</td> <td>12,97</td> <td>11,64</td> <td>16,88</td> </tr> <tr> <td>4</td> <td>4,08</td> <td>12,97</td> <td>11,64</td> <td>16,97</td> </tr> <tr> <td colspan="2">Error of last row</td> <td>0,5%</td> <td>0%</td> <td>0,3%</td> <td>0,1%</td> </tr> </tbody> </table> | Analytic results | e _z deflection [mm] | | | | Distributed load | | Concentrated load | | b/a=1 | b/a=5 | b/a=1 | b/a=5 | Clamped support | 1,26 | 2,56 | 5,60 | 7,23 | Simple support | 4,06 | 12,97 | 11,60 | 16,96 | Clamped support | | e _z deflection [mm] | | | | | | Distributed load | | Concentrated load | | | | b/a=1 | b/a=5 | b/a=1 | b/a=5 | Analytic results | | 1,26 | 2,56 | 5,60 | 7,23 | Mesh cases | 1 | 1,24 | 2,62 | 5,40 | 6,31 | 2 | 1,26 | 2,61 | 5,57 | 7,07 | 3 | 1,27 | 2,60 | 5,61 | 7,10 | 4 | 1,27 | 2,61 | 5,62 | 7,24 | Error of last row | | 0,8% | 1,9% | 0,4% | 0,1% | Simple support | | e _z deflection [mm] | | | | | | Distributed load | | Concentrated load | | | | b/a=1 | b/a=5 | b/a=1 | b/a=5 | Analytic results | | 4,06 | 12,97 | 11,60 | 16,96 | Mesh cases | 1 | 4,17 | 12,79 | 11,70 | 16,78 | 2 | 4,09 | 12,30 | 11,64 | 16,33 | 3 | 4,08 | 12,97 | 11,64 | 16,88 | 4 | 4,08 | 12,97 | 11,64 | 16,97 | Error of last row | | 0,5% | 0% | 0,3% | 0,1% |
| Analytic results | e _z deflection [mm] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Distributed load | | Concentrated load | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | b/a=1 | b/a=5 | b/a=1 | b/a=5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Clamped support | 1,26 | 2,56 | 5,60 | 7,23 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Simple support | 4,06 | 12,97 | 11,60 | 16,96 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Clamped support | | e _z deflection [mm] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Distributed load | | Concentrated load | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | b/a=1 | b/a=5 | b/a=1 | b/a=5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Analytic results | | 1,26 | 2,56 | 5,60 | 7,23 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Mesh cases | 1 | 1,24 | 2,62 | 5,40 | 6,31 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 2 | 1,26 | 2,61 | 5,57 | 7,07 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 3 | 1,27 | 2,60 | 5,61 | 7,10 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 4 | 1,27 | 2,61 | 5,62 | 7,24 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Error of last row | | 0,8% | 1,9% | 0,4% | 0,1% | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Simple support | | e _z deflection [mm] | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | Distributed load | | Concentrated load | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | b/a=1 | b/a=5 | b/a=1 | b/a=5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Analytic results | | 4,06 | 12,97 | 11,60 | 16,96 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Mesh cases | 1 | 4,17 | 12,79 | 11,70 | 16,78 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 2 | 4,09 | 12,30 | 11,64 | 16,33 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 3 | 4,08 | 12,97 | 11,64 | 16,88 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 4 | 4,08 | 12,97 | 11,64 | 16,97 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Error of last row | | 0,5% | 0% | 0,3% | 0,1% | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

| Clamped support distorted elements | | e_z deflection [mm] | | | |
|---------------------------------------|---|-----------------------|-------|-------------------|-------|
| | | Distributed load | | Concentrated load | |
| | | b/a=1 | b/a=5 | b/a=1 | b/a=5 |
| Analytic results | | 1,26 | 2,56 | 5,60 | 7,23 |
| Mesh cases | 5 | 1,00 | 2,98 | 3,95 | 2,50 |
| | 6 | 1,16 | 2,57 | 5,07 | 5,47 |
| | 7 | 1,25 | 2,60 | 5,51 | 5,81 |
| | 8 | 1,26 | 2,60 | 5,60 | 6,59 |
| Error of last row | | 0% | 1,5% | 0% | 9,7% |

| Simple support distorted elements | | e_z deflection [mm] | | | |
|--------------------------------------|---|-----------------------|-------|-------------------|-------|
| | | Distributed load | | Concentrated load | |
| | | b/a=1 | b/a=5 | b/a=1 | b/a=5 |
| Analytic results | | 4,06 | 12,97 | 11,60 | 16,96 |
| Mesh cases | 5 | 3,95 | 14,25 | 10,93 | 12,40 |
| | 6 | 4,07 | 12,95 | 11,36 | 13,83 |
| | 7 | 4,08 | 12,98 | 11,58 | 14,79 |
| | 8 | 4,08 | 12,97 | 11,63 | 16,15 |
| Error of last row | | 0,5% | 0% | 0,3% | 4,8% |